

# **HYDROLOGY STUDY**

For:

## **TORRANCE WAREHOUSE**

*Project Site Location/Address:*

**2555 W. 190<sup>th</sup> Street  
Torrance, CA 90504**

*Prepared For:*

**St. Paul Fire and Marine Insurance Co.,  
A Connecticut Corporation  
385 Washington Street  
St. Paul, MN 55102**

*Lead Agency:*

**City of Torrance  
3031 Torrance Blvd.  
Torrance, CA 90503**

*Prepared by:*

**DRC Engineering, Inc.  
160 S. Old Springs Road, Suite 210  
Anaheim, CA 92808  
(714) 685-6860  
Matthew Helleesen, P.E.**

**July 29, 2020**

Project No. 19-040

**TABLE OF CONTENTS**

<b>SECTION 1.0</b>	<b>NARRATIVE</b> Introduction Project Description Hydrology Analysis Summary & Conclusions
<b>SECTION 2.0</b>	<b>SITE INFORMATION</b> Location Map LA County Hydrology Map
<b>SECTION 3.0</b>	<b>EXISTING SITE HYDROLOGIC ANALYSIS</b> Existing Hydrology Map 10-year Storm Calculations 50-year Storm Calculations
<b>SECTION 4.0</b>	<b>PROPOSED SITE HYDROLOGIC ANALYSIS</b> Proposed Hydrology Map 10-year Storm Calculations 50-year Storm Calculations Detention System and Outlet Control Calculations
<b>SECTION 5.0</b>	<b>CITY OF TORRANCE</b> Storm Drain Master Plan Tributary Areas Q and Pipe Capacity Data

## ***Introduction***

The proposed project is located at 2555 W. 190<sup>th</sup> Street in the City of Torrance, California.

The project is 14.2 acres, with approximately 291,000 square feet in building area. The proposed building will be a manufacturing warehouse and surrounded by paved parking and landscape. An existing building and parking will be demolished and regrading of the site will occur.

This report has been prepared to calculate the post-construction hydrologic conditions for peak storm runoff rates and demonstrate the overall impact to the existing drainage infrastructure as well as demonstrate that the proposed project will not exceed the existing condition.

## ***Project Description***

### **Existing Site Conditions:**

The project site is 14.23 acres. At its existing state, approximately 1.26 acres is building, 11.38 acres is impervious pavement, and 1.59 acres is landscaping (11.1% pervious).

Per the existing hydrology map, 8.65 acres (DA-1) is sheet flowing southwesterly towards and out of the corner of the property, eventually draining to the street catch basin at the northeast corner of 190<sup>th</sup> Street and Crenshaw Place. 5.58 acres (DA-2) of runoff flows towards individual inlets throughout the area and collects into an underground storm drain network. The laterals eventually collect into a 27-inch storm drain main that runs from north to south, connecting to a county storm drain RCB at 190<sup>th</sup> Street.

As part of the study, approximately 2.69 acres of off-site area (DA-0) located north of project site is included due to its drainage runoff contributing to the 27-inch storm drain.

*Reference Table 1.1 and Section 3.0 for Existing Hydrology Map*

### **Proposed Site Conditions:**

The proposed site will see the demolition of the existing building, pavement, and landscaping and the development of 291,000 square feet (6.68 acres) of a manufacturing warehouse, 6.21 acres of parking/sidewalk (impervious), and 1.34 acres of landscaping.

Per the proposed hydrology map, all 14.32 acres of development will flow towards individual inlets and eventually collects into underground piping and outlets into the existing 6'-9" x 8' LACFCD RCB.

As part of the study, approximately 2.69 acres of off-site area (DA-0) located north of project site is included due to its drainage runoff contributing to the existing 27-inch storm drain, which will be upsized and relocated easterly due to the location of the proposed warehouse.

*Reference Tables 1.2 and 1.3 and Section 4.0 for Proposed Hydrology Map*



## ***Hydrology Methodology***

### **Methodology**

For both existing and proposed conditions, the peak storm discharge for the drainage sub-areas was calculated based on the Los Angeles County Department of Public Works (LADPW) Hydrology Manual. LA County Hydrocalc spreadsheets (see Section 3.0 for existing and Section 4.0 for proposed) were used to calculate the 10-year and 50-year storm events. Per the LA County Hydrology Map, the soil on-site is type 09 and the 50-year isohyet is 5.8 (see Section 2.0). Due to the peak runoff for each of the above storm events for the proposed conditions exceeding the peak runoff of the existing conditions, hydromodification is required.

The Hydrocalc spreadsheets were also used to calculate the unit hydrographs for the 10-year and 50-year storm events both existing and proposed. The peak runoff from each event in the proposed condition was then input into the Hydraflow Hydrograph program (an extension of Civil 3D) to design the outlet control and detention storage.

The hydrology analysis was done for the property as well as the off-site property to the north. Note: Drainage areas DA-0 and DA-3 in the proposed condition will not be included as part of the unit hydrology analysis and detention design/outlet control (i.e., only DA-1 and DA-2 of the proposed condition will be used).

### **Existing Condition:**

The existing project site peak runoff rates are determined using the LA County Hydrocalc spreadsheets. Refer to Section 3.0, the Existing Condition Hydrology Calculations of this report for data used in the calculations. The following table illustrates the runoff rate calculation results for the existing conditions. These values are used as the benchmark for flow reduction in the post-developed (proposed) condition for the on-site areas.

**Table 1.1: EXISTING DRAINAGE SUMMARY TABLE**

<b>Drainage Area</b>	<b>Area (ac.)</b>	<b>% Impervious</b>	<b>FLOWRATE: 10-YR STORM (cfs)</b>	<b>FLOWRATE: 50-YR STORM (cfs)</b>
DA-0	2.69	90	5.94	8.38
DA-1	8.65	98	12.22	18.57
DA-2	5.58	74	12.17	17.38
<b>TOTAL</b>	<b>16.92</b>		<b>30.33</b>	<b>44.33</b>

### **Proposed Condition:**

*Reference Tables 1.2 and 1.3 and Section 4.0 for Proposed Hydrology Map*

The proposed site ultimately discharges to an existing 6'-9"x8' LACFCD RCB. With a fully developed site, the total peak flow rates from each storm event exceeds the existing peak flow rate, as shown in Table 1.2. To maintain or be less than the existing total outflow, underground detention systems are placed at drainage areas DA-1 and DA-2. Per Table 1.3, comparing the flows to that of the existing condition, the post-developed total on-site runoff rates are lower

due to the utilization of the underground detention systems. In addition, prior to flowing off-site, site storm water runoff will be treated by a combination of biofiltration basins and other proprietary water quality systems so as to remove, to acceptable levels, the pollutants of concern generated by the project and the pollutants of concerns for the downstream watercourses.

**Table 1.2: PROPOSED DRAINAGE SUMMARY TABLE**

Drainage Area	Area (ac.)	% Impervious	FLOWRATE: 10-YR STORM (cfs)	FLOWRATE: 50-YR STORM (cfs)
DA-0	2.69	90	5.94	8.38
DA-1	6.38	86	14.04	21.96
DA-2	7.05	95	15.62	19.87
DA-3	0.8	89	1.76	2.49
<b>TOTAL</b>	<b>16.92</b>		<b>37.36</b>	<b>52.70</b>

**Table 1.3: PROPOSED DRAINAGE SUMMARY TABLE W/ DET. SYSTEM (DA-1 & DA-2 ONLY)**

Drainage Area	Area (ac.)	% Impervious	FLOWRATE: 10-YR STORM (cfs)	FLOWRATE: 50-YR STORM (cfs)
DA-0	2.69	90	5.94	8.38
DA-1	6.38	86	8.54	16.53
DA-2	7.05	95	7.91	15.90
DA-3	0.8	89	1.76	2.49
<b>TOTAL</b>	<b>16.92</b>		<b>24.15</b>	<b>43.30</b>

DETENTION A  
DETENTION B



## ***Summary and Conclusions***

Refer to sections 3 & 4 for the proposed and existing hydrology maps depicting the drainage areas of concern. Analyzing the effective area, it is computed that the total existing site flowrate in the 10-year and 50-year is **30.33 cfs** and **44.33 cfs**, respectively, and the proposed site flowrate is **24.15 cfs** and **43.30 cfs** when detention systems are in place.

Through the implementation of the detention systems and outlet controls, the peak discharges for the 10-year and 50-year storm events were dropped to below existing condition levels.

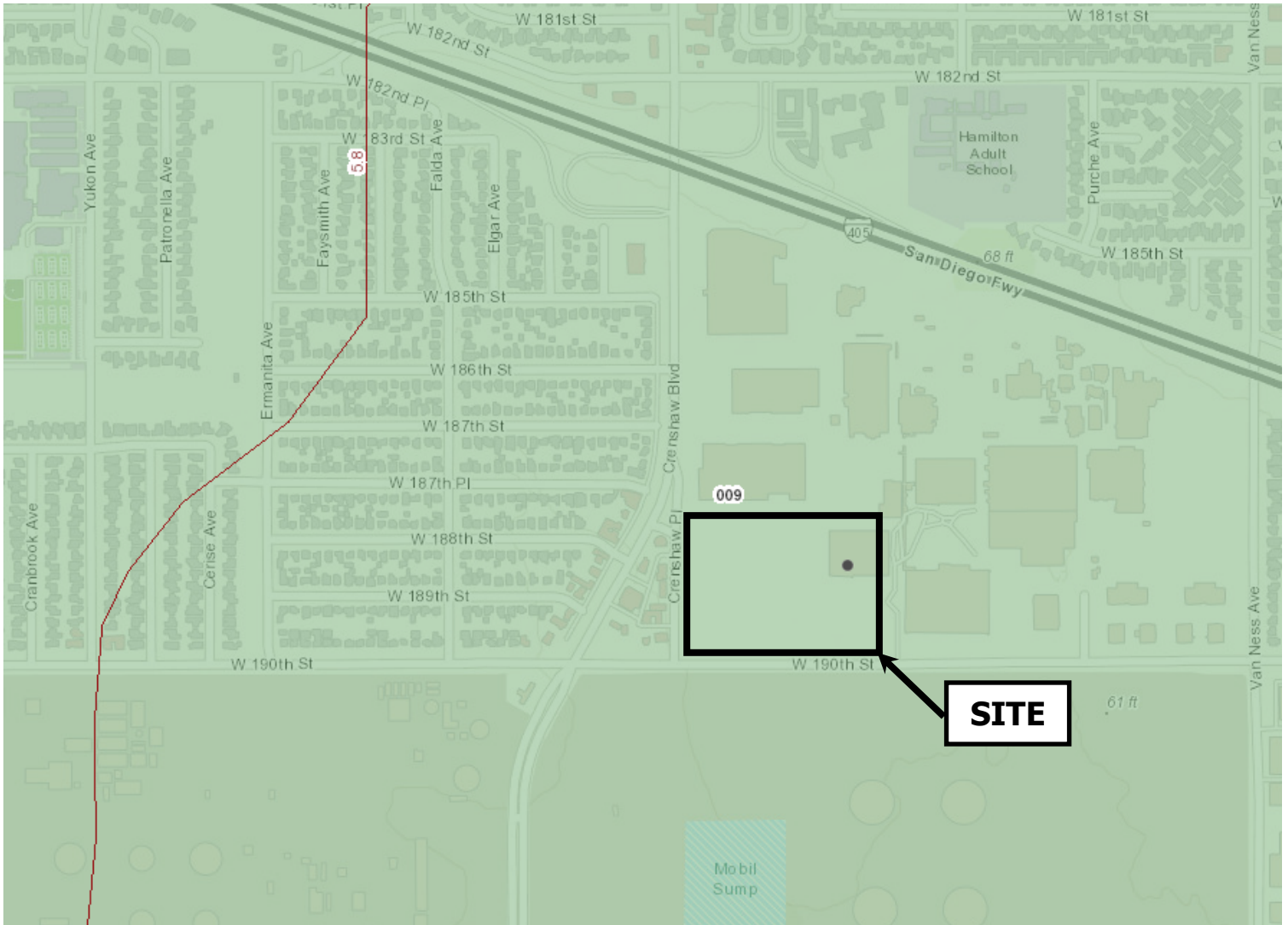
**SECTION 2.0**

VICINITY MAP  
(NOT TO SCALE)



LA COUNTY HYDROLOGY MAP  
(NOT TO SCALE)

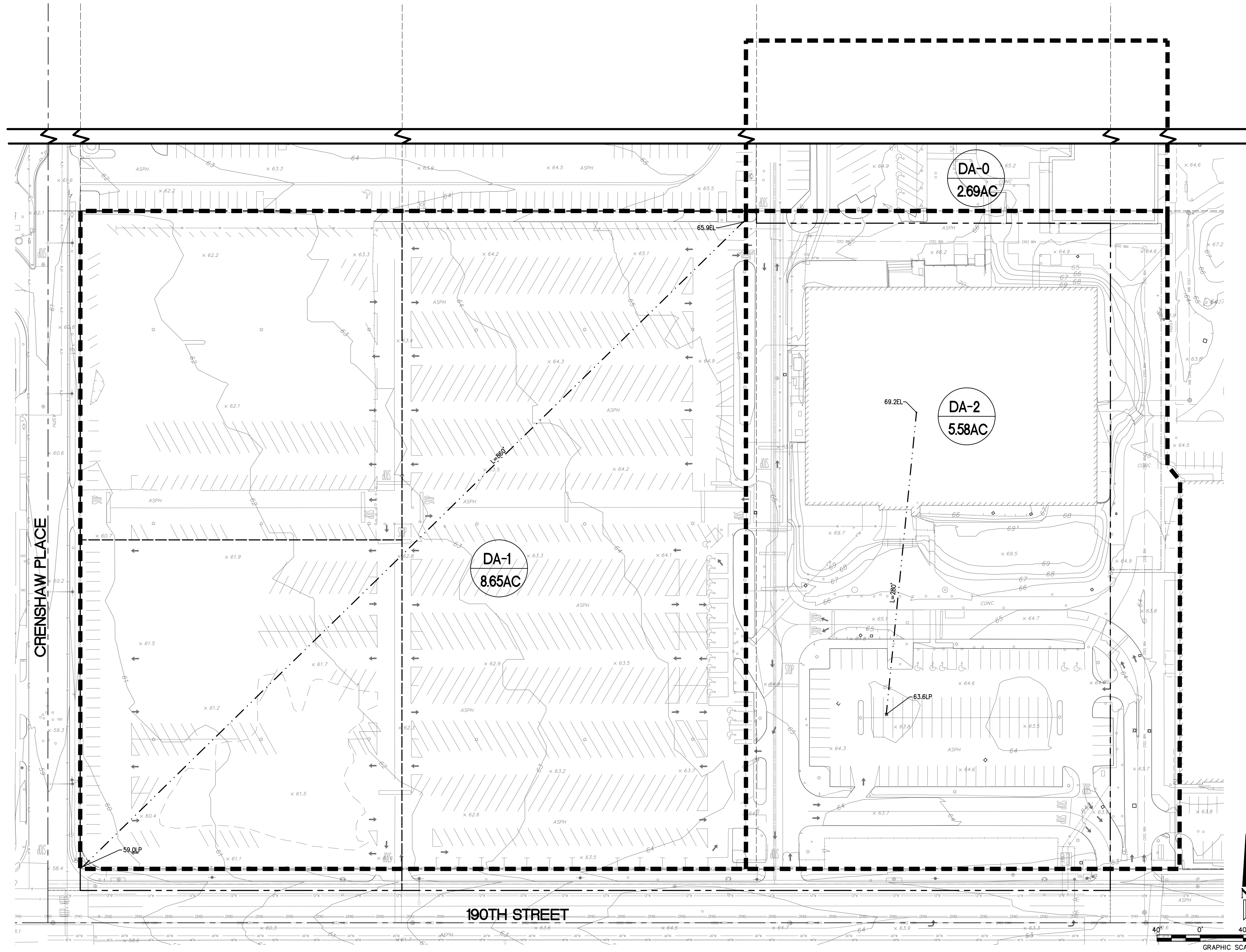
LA County Hydrology Map





## **SECTION 3.0**

Existing Hydrology Map  
Existing 10-year Storm Calculations  
Existing 50-year Storm Calculations



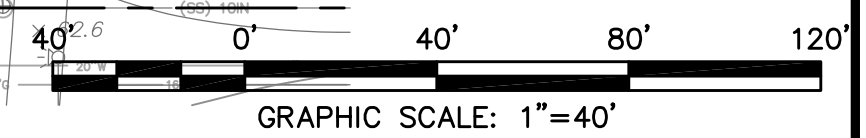
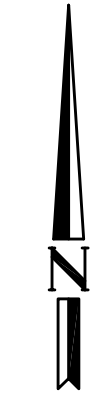
CRENSHAW PLACE


190TH STREET

DA-1  
8.65AC

DA-0  
2.69AC

DA-2  
5.58AC



PREPARED BY:  
  
 DORC Engineering, Inc.  
 Civil Engineering/Land Surveying/Land Planning  
 160 S. Old Springs Road  
 Suite 210  
 Anaheim Hills, CA 92808  
 714-685-6860

NO.	REVISION:

NO.	REVISION:	DATE:

PROJECT:  
**COMSTOCK TORRANCE**  
**2555 W. 190TH STREET**  
**TORRANCE, CALIFORNIA**

DRAWING NAME:  
**EXISTING HYDROLOGY MAP**

ISSUE:	HYDROLOGY
DATE:	2020-07-20
CHECKED: MH	DRAWN: HM
DRAWING FILE:	
PROJECT NO.:	<b>19-040</b>
SHEET NUMBER:	<b>1</b>
OF 2 SHEETS	
SCALE:	<b>AS SHOWN</b>

## Peak Flow Hydrologic Analysis

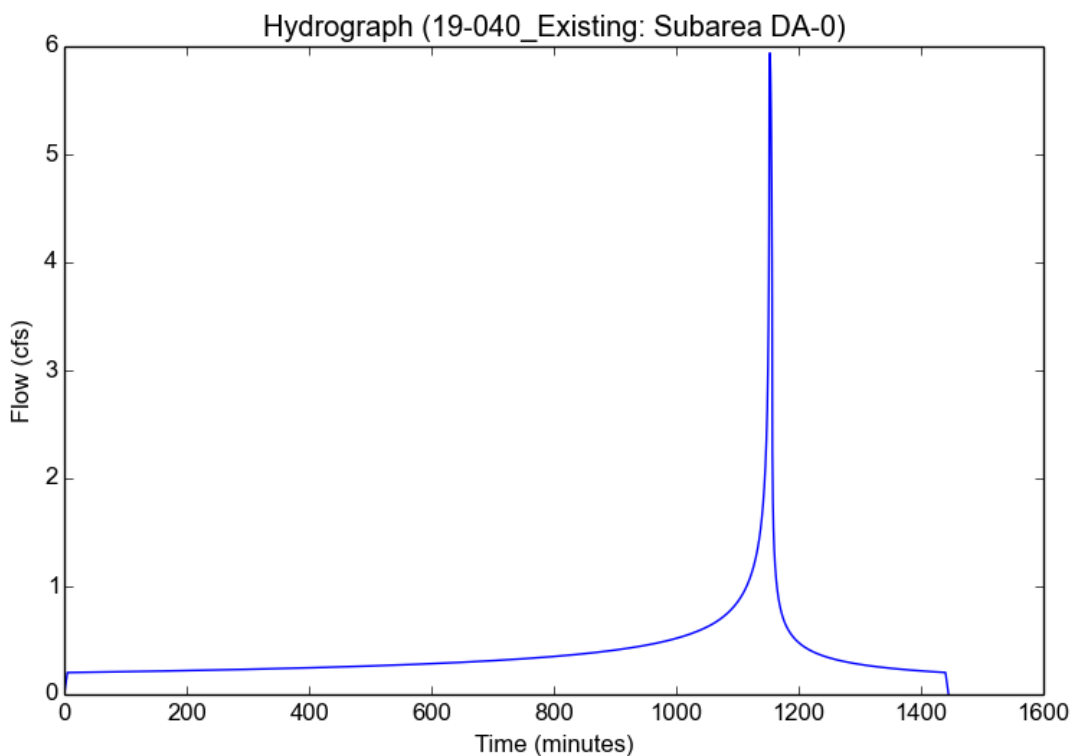
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19-040_Existing
Subarea ID	Subarea DA-0
Area (ac)	2.69
Flow Path Length (ft)	150.0
Flow Path Slope (vft/hft)	0.015
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.9
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

### Output Results

Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8934
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	5.938
Burned Peak Flow Rate (cfs)	5.938
24-Hr Clear Runoff Volume (ac-ft)	0.7617
24-Hr Clear Runoff Volume (cu-ft)	33177.4748



## Peak Flow Hydrologic Analysis

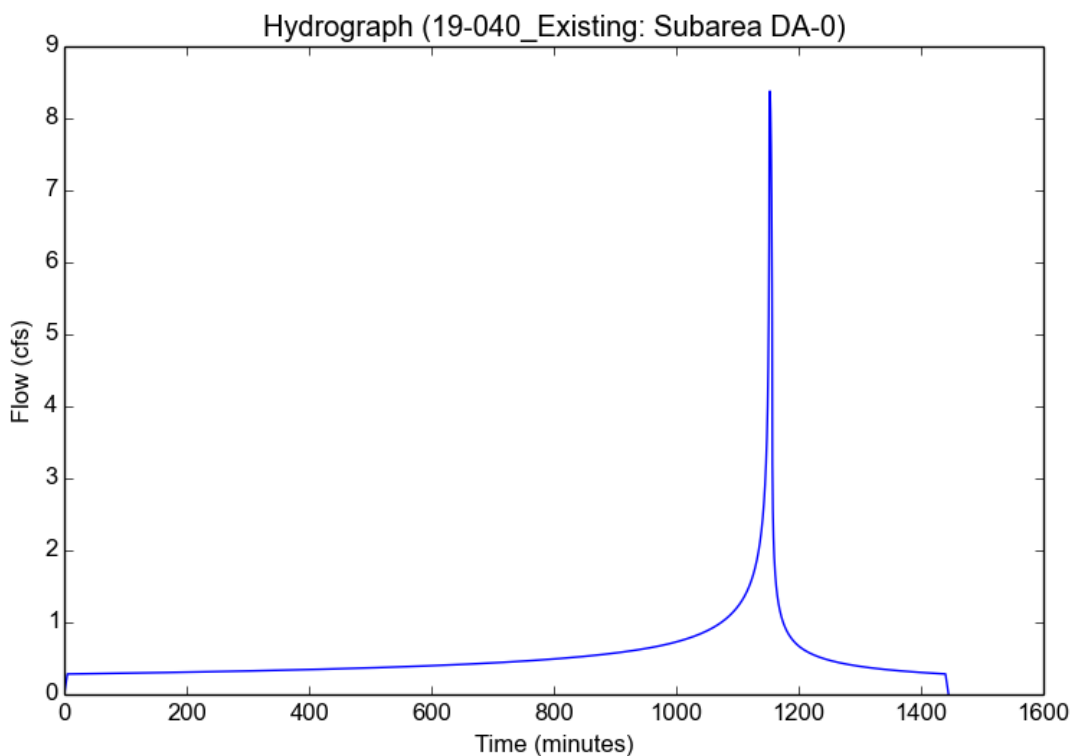
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19-040_Existing
Subarea ID	Subarea DA-0
Area (ac)	2.69
Flow Path Length (ft)	150.0
Flow Path Slope (vft/hft)	0.015
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.9
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

### Output Results

Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	8.3777
Burned Peak Flow Rate (cfs)	8.3777
24-Hr Clear Runoff Volume (ac-ft)	1.0712
24-Hr Clear Runoff Volume (cu-ft)	46661.3509



## Peak Flow Hydrologic Analysis

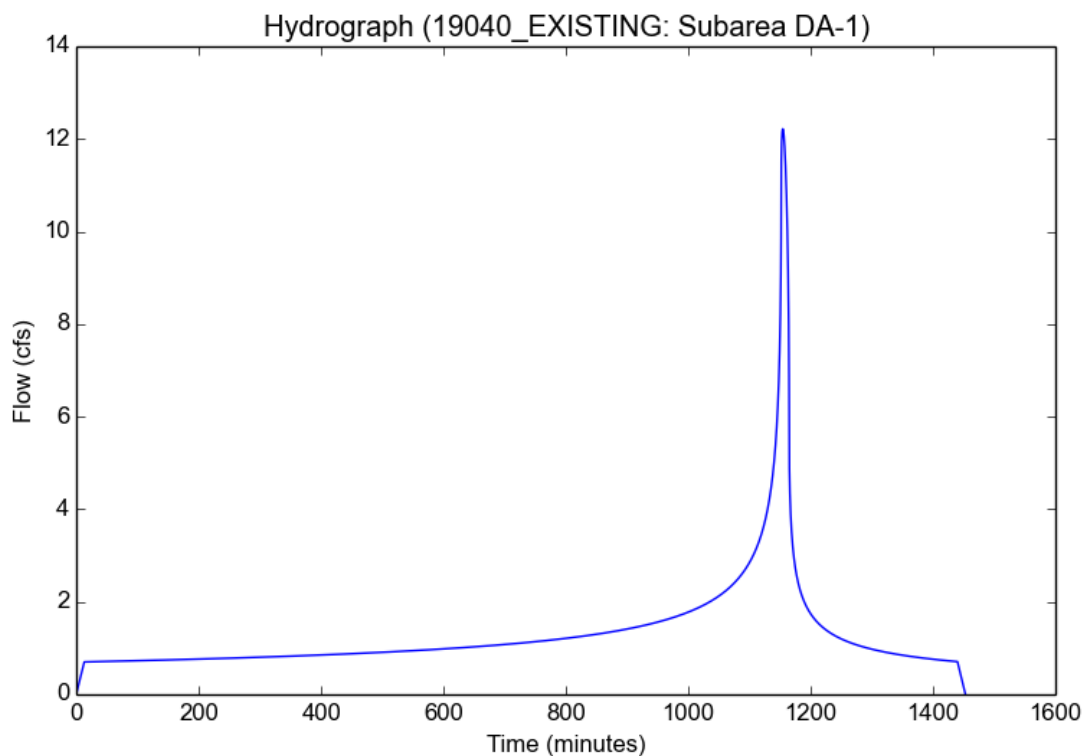
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_EXISTING
Subarea ID	Subarea DA-1
Area (ac)	8.65
Flow Path Length (ft)	860.0
Flow Path Slope (vft/hft)	0.007
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.98
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

### Output Results

Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	1.5769
Undeveloped Runoff Coefficient (Cu)	0.6878
Developed Runoff Coefficient (Cd)	0.8958
Time of Concentration (min)	13.0
Clear Peak Flow Rate (cfs)	12.2179
Burned Peak Flow Rate (cfs)	12.2179
24-Hr Clear Runoff Volume (ac-ft)	2.6212
24-Hr Clear Runoff Volume (cu-ft)	114178.1321



## Peak Flow Hydrologic Analysis

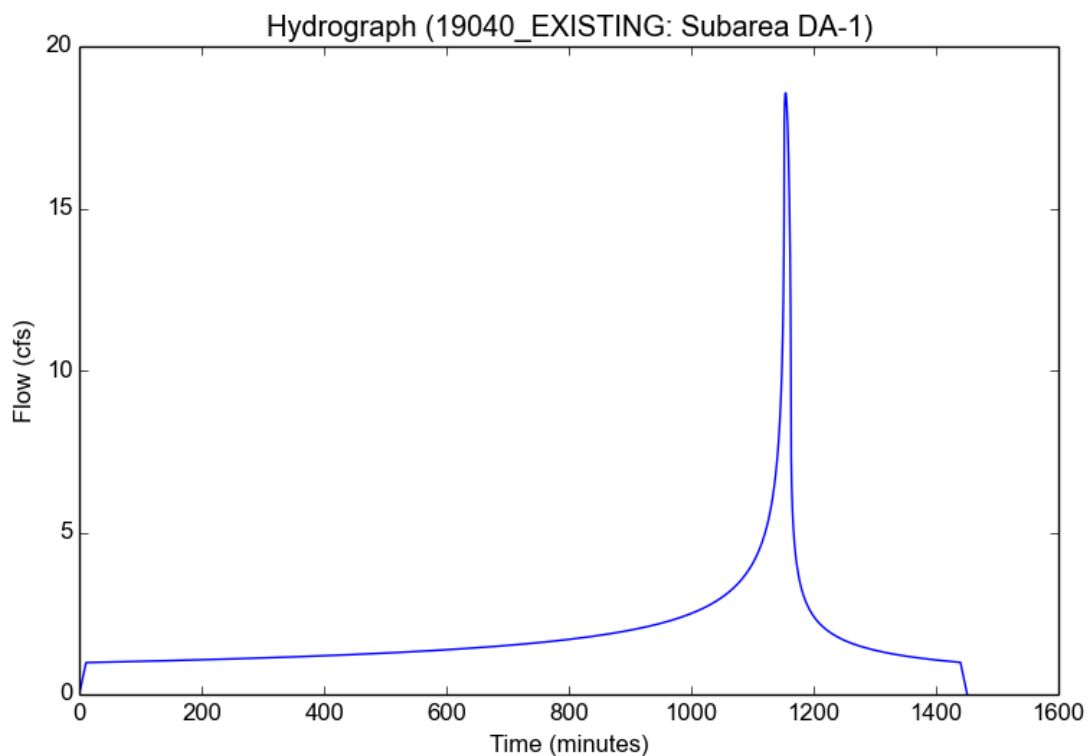
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_EXISTING
Subarea ID	Subarea DA-1
Area (ac)	8.65
Flow Path Length (ft)	860.0
Flow Path Slope (vft/hft)	0.007
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.98
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

### Output Results

Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	2.3889
Undeveloped Runoff Coefficient (Cu)	0.8226
Developed Runoff Coefficient (Cd)	0.8985
Time of Concentration (min)	11.0
Clear Peak Flow Rate (cfs)	18.5654
Burned Peak Flow Rate (cfs)	18.5654
24-Hr Clear Runoff Volume (ac-ft)	3.6741
24-Hr Clear Runoff Volume (cu-ft)	160045.7527



## Peak Flow Hydrologic Analysis

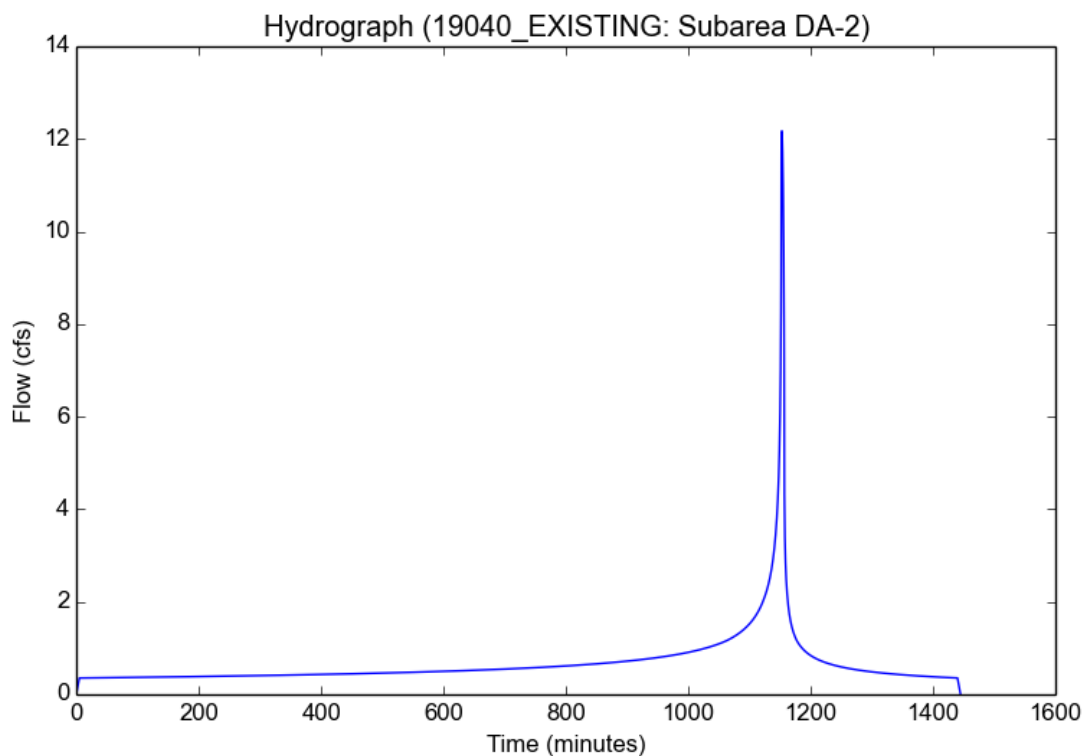
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_EXISTING
Subarea ID	Subarea DA-2
Area (ac)	5.58
Flow Path Length (ft)	280.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.74
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

### Output Results

Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8829
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	12.1725
Burned Peak Flow Rate (cfs)	12.1725
24-Hr Clear Runoff Volume (ac-ft)	1.3578
24-Hr Clear Runoff Volume (cu-ft)	59144.9883



## Peak Flow Hydrologic Analysis

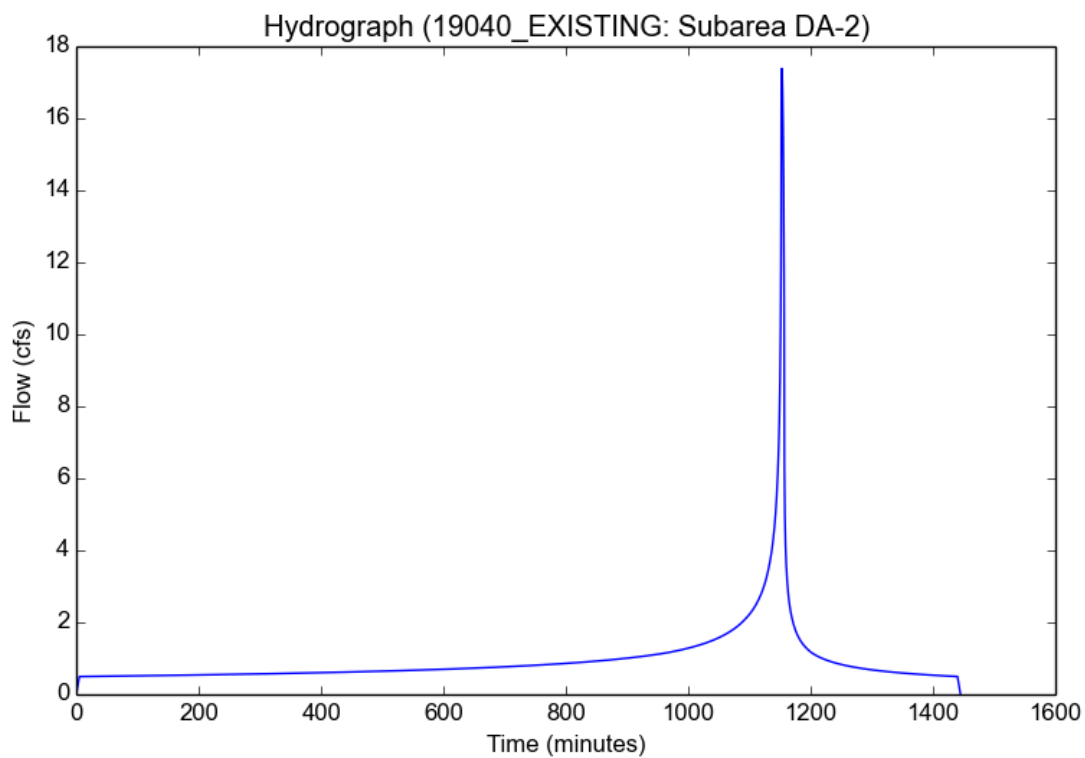
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### Input Parameters

Project Name	19040_EXISTING
Subarea ID	Subarea DA-2
Area (ac)	5.58
Flow Path Length (ft)	280.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.74
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

### Output Results

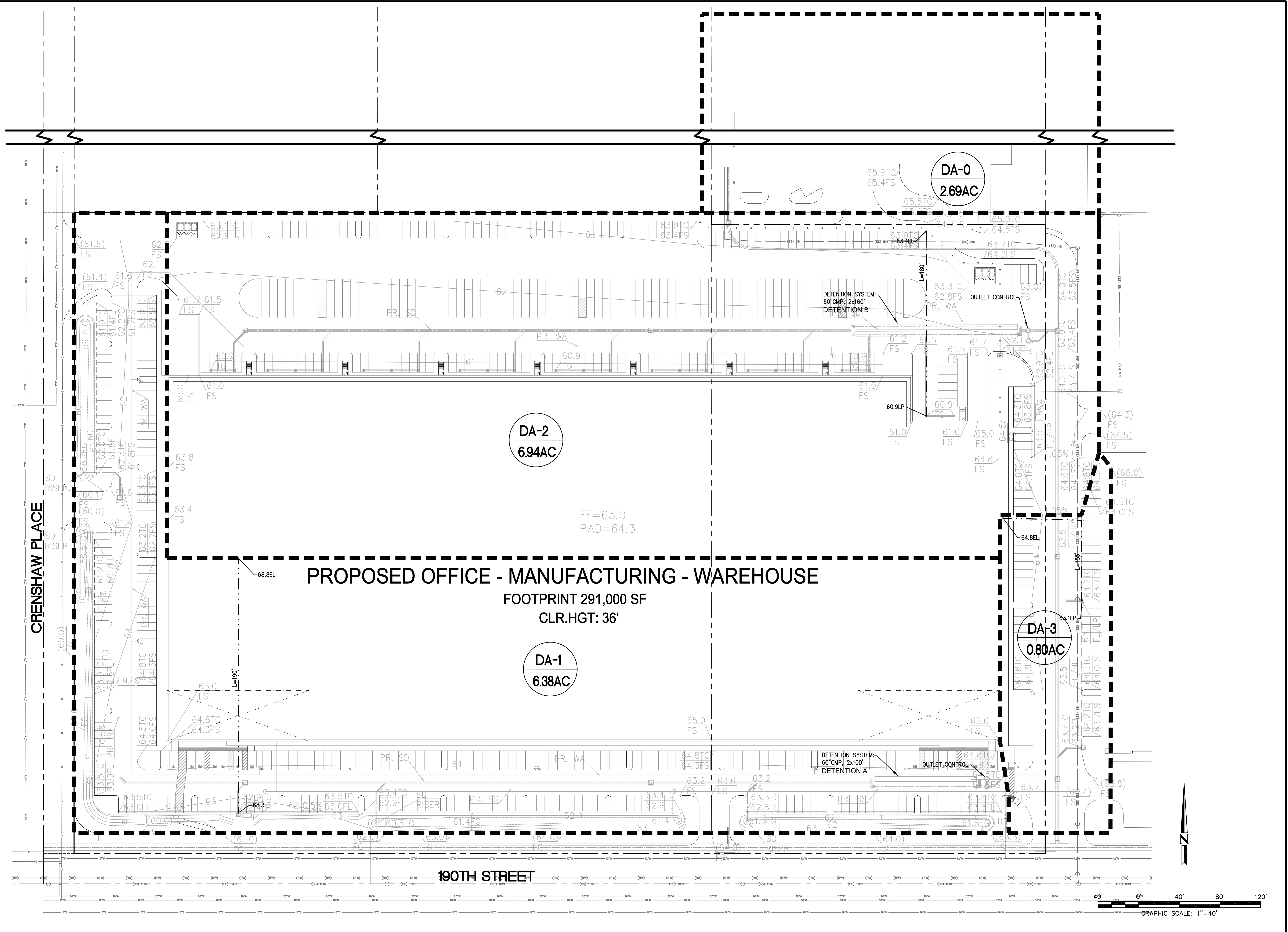
Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	17.3783
Burned Peak Flow Rate (cfs)	17.3783
24-Hr Clear Runoff Volume (ac-ft)	1.9257
24-Hr Clear Runoff Volume (cu-ft)	83884.0343





## **SECTION 4.0**

Proposed Hydrology Map  
Proposed 10-year Storm Calculations  
Proposed 50-year Storm Calculations  
Detention System and Outlet Control Calculations



DA-2  
6.94AC

DA-1  
6.38AC

DA-0  
2.69AC

DA-3  
0.80AC

## PROPOSED OFFICE - MANUFACTURING - WAREHOUSE

FOOTPRINT 291,000 SF  
CLR.HGT: 36'

FF=65.0  
PAD=64.3

PREPARED BY:  
**DMRC** Engineering, Inc.  
Civil Engineering/Land Surveying/Land Planning  
160 S. Old Springs Road  
Suite 210  
Anaheim Hills, CA 92808  
714-685-6860

NO.	REVISION:	DATE:

PROJECT:  
**COMSTOCK TORRANCE  
2555 W. 190TH STREET  
TORRANCE, CALIFORNIA**

DRAWING NAME:  
**PROPOSED HYDROLOGY MAP**

ISSUE:	HYDROLOGY
DATE:	2020-07-20
CHECKED:	MH DRAWN: HM
DRAWING FILE:	
PROJECT NO.:	10-040
SHEET NUMBER:	2
OF 2 SHEETS	
SCALE:	AS SHOWN

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NOT FOR CONSTRUCTION

## Peak Flow Hydrologic Analysis

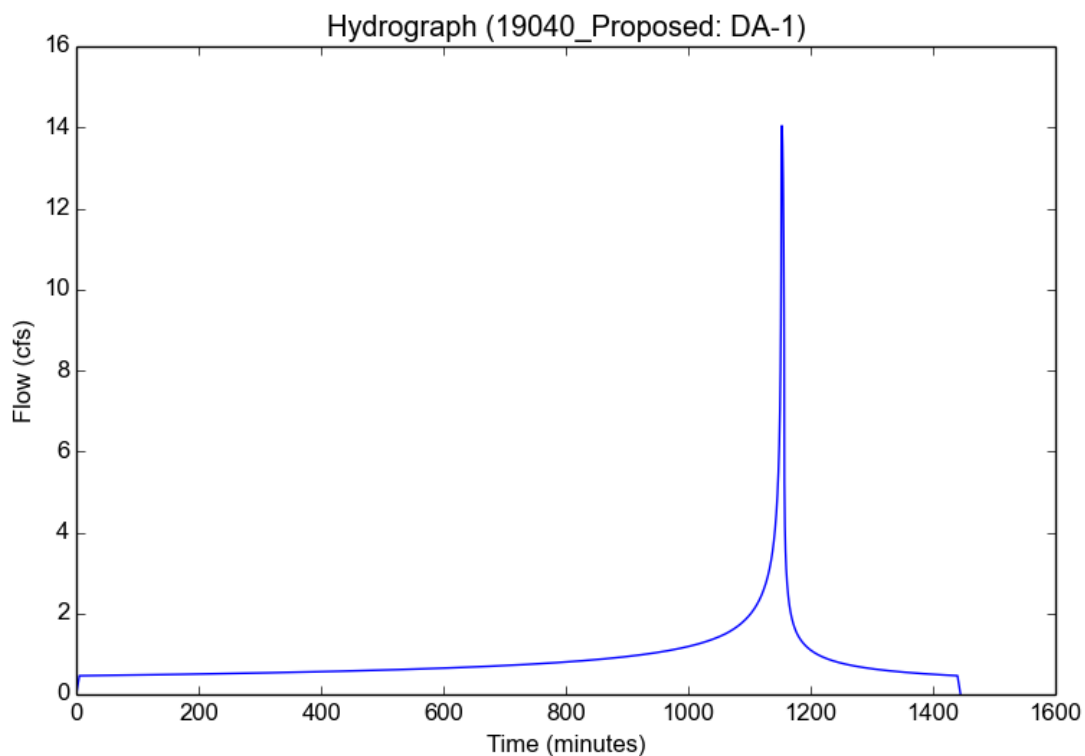
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_Proposed
Subarea ID	DA-1
Area (ac)	6.38
Flow Path Length (ft)	190.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.86
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

### Output Results

Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8908
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	14.042
Burned Peak Flow Rate (cfs)	14.042
24-Hr Clear Runoff Volume (ac-ft)	1.7429
24-Hr Clear Runoff Volume (cu-ft)	75922.5767



## Peak Flow Hydrologic Analysis

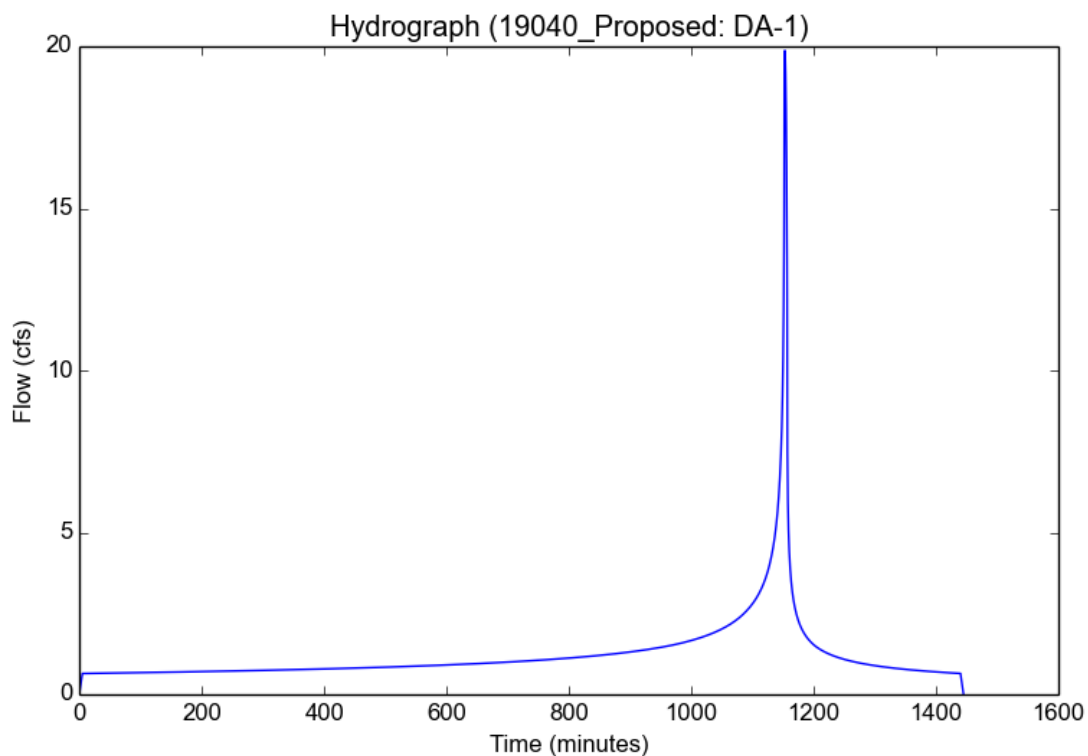
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_Proposed
Subarea ID	DA-1
Area (ac)	6.38
Flow Path Length (ft)	190.0
Flow Path Slope (vft/hft)	0.02
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.86
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

### Output Results

Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	19.8698
Burned Peak Flow Rate (cfs)	19.8698
24-Hr Clear Runoff Volume (ac-ft)	2.4559
24-Hr Clear Runoff Volume (cu-ft)	106979.3015



## Peak Flow Hydrologic Analysis

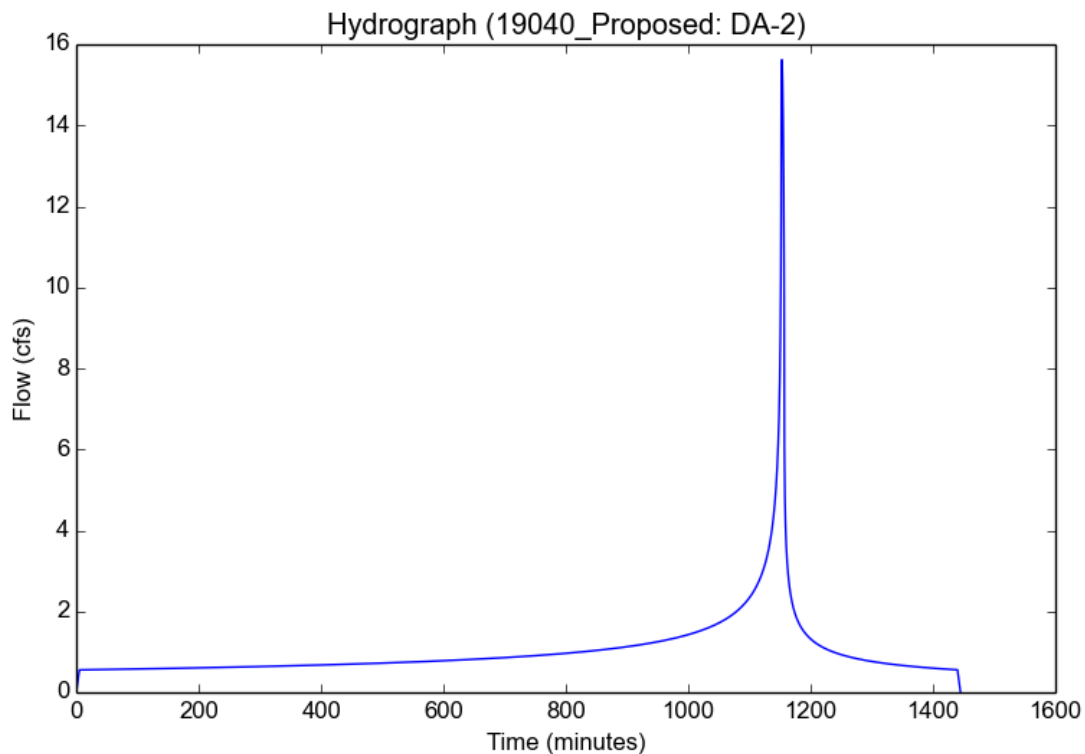
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_Proposed
Subarea ID	DA-2
Area (ac)	7.05
Flow Path Length (ft)	180.0
Flow Path Slope (vft/hft)	0.014
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.95
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

### Output Results

Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8967
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	15.6197
Burned Peak Flow Rate (cfs)	15.6197
24-Hr Clear Runoff Volume (ac-ft)	2.0839
24-Hr Clear Runoff Volume (cu-ft)	90772.7191



## Peak Flow Hydrologic Analysis

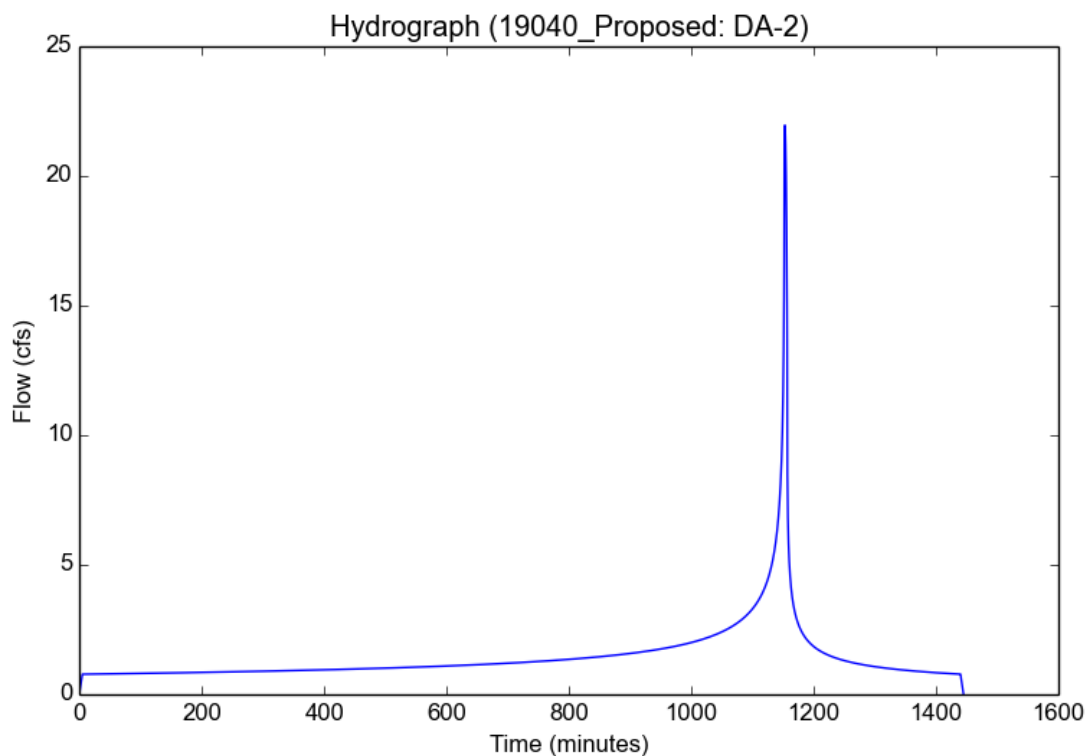
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_Proposed
Subarea ID	DA-2
Area (ac)	7.05
Flow Path Length (ft)	180.0
Flow Path Slope (vft/hft)	0.014
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.95
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

### Output Results

Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	21.9565
Burned Peak Flow Rate (cfs)	21.9565
24-Hr Clear Runoff Volume (ac-ft)	2.9244
24-Hr Clear Runoff Volume (cu-ft)	127387.2713



## Peak Flow Hydrologic Analysis

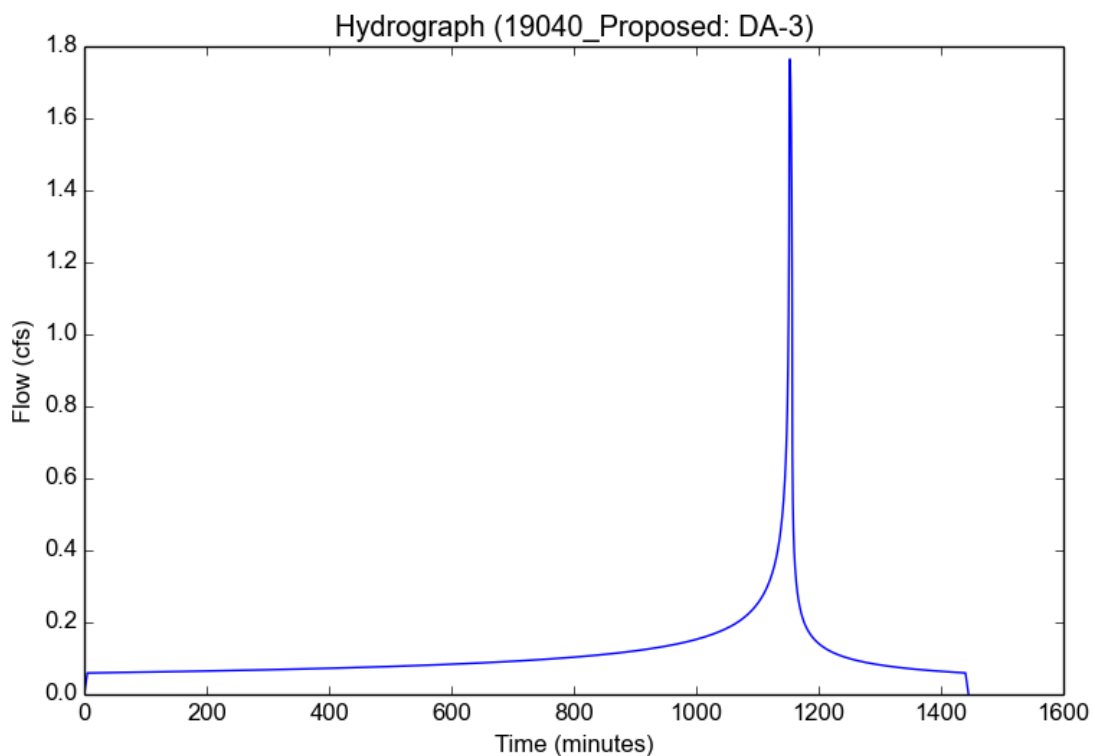
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Version: HydroCalc 1.0.2

### Input Parameters

Project Name	19040_Proposed
Subarea ID	DA-3
Area (ac)	0.8
Flow Path Length (ft)	155.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.89
Soil Type	9
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

### Output Results

Modeled (10-yr) Rainfall Depth (in)	4.1412
Peak Intensity (in/hr)	2.4708
Undeveloped Runoff Coefficient (Cu)	0.8343
Developed Runoff Coefficient (Cd)	0.8928
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	1.7647
Burned Peak Flow Rate (cfs)	1.7647
24-Hr Clear Runoff Volume (ac-ft)	0.2245
24-Hr Clear Runoff Volume (cu-ft)	9780.1983



# Peak Flow Hydrologic Analysis

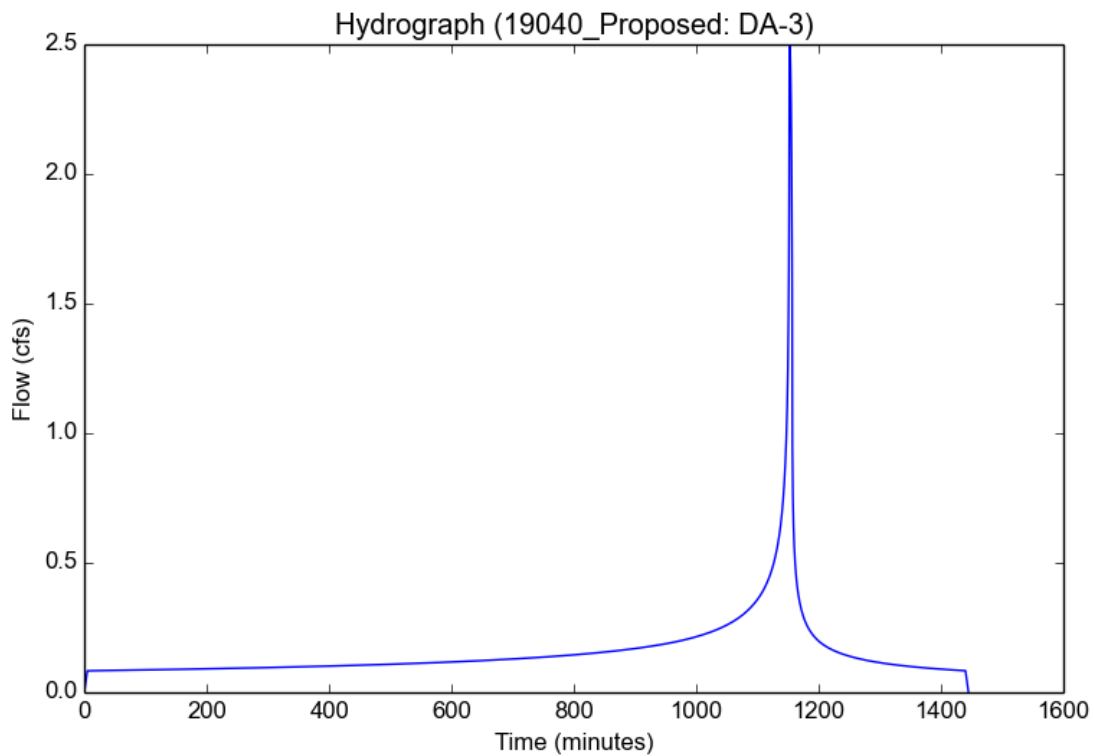
File location: M:/2019/19-040 Comstock Torrance/hm/hydrocalc/19040\_Proposed - DA-3\_50yr.pdf  
Version: HydroCalc 1.0.2

## Input Parameters

Project Name	19040_Proposed
Subarea ID	DA-3
Area (ac)	0.8
Flow Path Length (ft)	155.0
Flow Path Slope (vft/hft)	0.01
50-yr Rainfall Depth (in)	5.8
Percent Impervious	0.89
Soil Type	9
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

## Output Results

Modeled (50-yr) Rainfall Depth (in)	5.8
Peak Intensity (in/hr)	3.4604
Undeveloped Runoff Coefficient (Cu)	0.9013
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	5.0
Clear Peak Flow Rate (cfs)	2.4915
Burned Peak Flow Rate (cfs)	2.4915
24-Hr Clear Runoff Volume (ac-ft)	0.3159
24-Hr Clear Runoff Volume (cu-ft)	13761.3194





# Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	14.04	5	1160	76,302	-----	-----	-----	Prop DA-1_10yr
2	Reservoir	8.537	5	1165	76,300	1	52.00	3,269	Detention A
4	Manual	15.62	5	1160	91,200	-----	-----	-----	Prop DA-2_10yr
5	Reservoir	7.909	5	1165	91,197	4	54.20	5,103	Detention B
7	Manual	1.760	5	1160	9,816	-----	-----	-----	Prop DA-3_10yr
10	Manual	0.000	5	n/a	0	-----	-----	-----	Prop DA-1_50yr
11	Reservoir	0.000	5	n/a	0	10	46.90	0.000	Detention A
13	Manual	0.000	5	n/a	0	-----	-----	-----	Prop DA-2_50yr
14	Reservoir	0.000	5	n/a	0	13	49.81	0.000	Detention B
16	Manual	0.000	5	n/a	0	-----	-----	-----	Prop DA-3_50yr

10-year

# Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Manual	0.000	5	n/a	0	-----	-----	-----	Prop DA-1_10yr
2	Reservoir	0.000	5	n/a	0	1	46.90	0.000	Detention A
4	Manual	0.000	5	n/a	0	-----	-----	-----	Prop DA-2_10yr
5	Reservoir	0.000	5	n/a	0	4	49.81	0.000	Detention B
7	Manual	0.000	5	n/a	0	-----	-----	-----	Prop DA-3_10yr
10	Manual	19.87	5	1160	107,514	-----	-----	-----	Prop DA-1_50yr
11	Reservoir	16.53	5	1160	107,512	10	52.00	3,663	Detention A
13	Manual	21.96	5	1160	127,965	-----	-----	-----	Prop DA-2_50yr
14	Reservoir	15.90	5	1160	127,962	13	54.97	5,788	Detention B
16	Manual	2.490	5	1160	13,830	-----	-----	-----	Prop DA-3_50yr

50-year

## Pond No. 1 - Detention A

### Pond Data

UG Chambers -Invert elev. = 46.90 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 100.00 ft, No. Barrels = 2, Slope = 0.10%, Headers = No

### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	46.90	n/a	0	0
0.51	47.41	n/a	181	181
1.02	47.92	n/a	355	536
1.53	48.43	n/a	438	974
2.04	48.94	n/a	484	1,458
2.55	49.45	n/a	507	1,964
3.06	49.96	n/a	507	2,471
3.57	50.47	n/a	485	2,956
4.08	50.98	n/a	437	3,392
4.59	51.49	n/a	354	3,747
5.10	52.00	n/a	181	3,928

### Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 12.00	0.00	0.00	0.00
Span (in)	= 12.00	0.00	0.00	0.00
No. Barrels	= 1	0	0	0
Invert El. (ft)	= 46.90	0.00	0.00	0.00
Length (ft)	= 1.00	0.00	0.00	0.00
Slope (%)	= 1.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

### Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 4.00	0.00	0.00	0.00
Crest El. (ft)	= 50.60	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

### Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	46.90	0.00	---	---	---	0.00	---	---	---	---	---	0.000
0.51	181	47.41	0.26 oc	---	---	---	0.00	---	---	---	---	---	0.261
1.02	536	47.92	0.88 oc	---	---	---	0.00	---	---	---	---	---	0.882
1.53	974	48.43	3.74 oc	---	---	---	0.00	---	---	---	---	---	3.743
2.04	1,458	48.94	4.69 ic	---	---	---	0.00	---	---	---	---	---	4.692
2.55	1,964	49.45	5.41 ic	---	---	---	0.00	---	---	---	---	---	5.414
3.06	2,471	49.96	6.05 ic	---	---	---	0.00	---	---	---	---	---	6.050
3.57	2,956	50.47	6.63 ic	---	---	---	0.00	---	---	---	---	---	6.625
4.08	3,392	50.98	7.15 ic	---	---	---	3.12	---	---	---	---	---	10.27
4.59	3,747	51.49	7.65 ic	---	---	---	11.18	---	---	---	---	---	18.83
5.10	3,928	52.00	8.11 ic	---	---	---	22.06	---	---	---	---	---	30.17

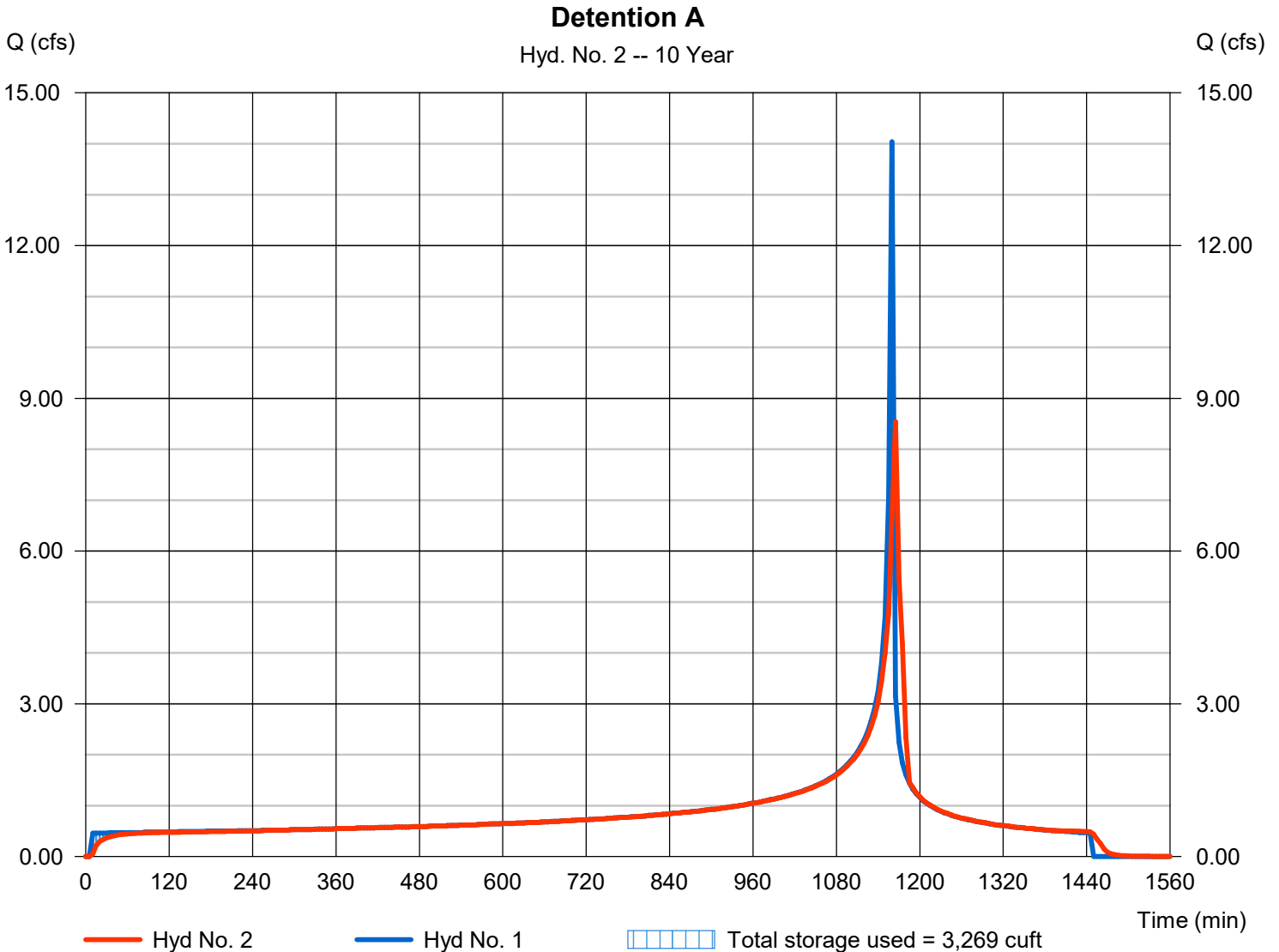
# Hydrograph Report

## Hyd. No. 2

### Detention A

Hydrograph type	= Reservoir	Peak discharge	= 8.537 cfs
Storm frequency	= 10 yrs	Time to peak	= 1165 min
Time interval	= 5 min	Hyd. volume	= 76,300 cuft
Inflow hyd. No.	= 1 - Prop DA-1_10yr	Max. Elevation	= 52.00 ft
Reservoir name	= Detention A	Max. Storage	= 3,269 cuft

Storage Indication method used.



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

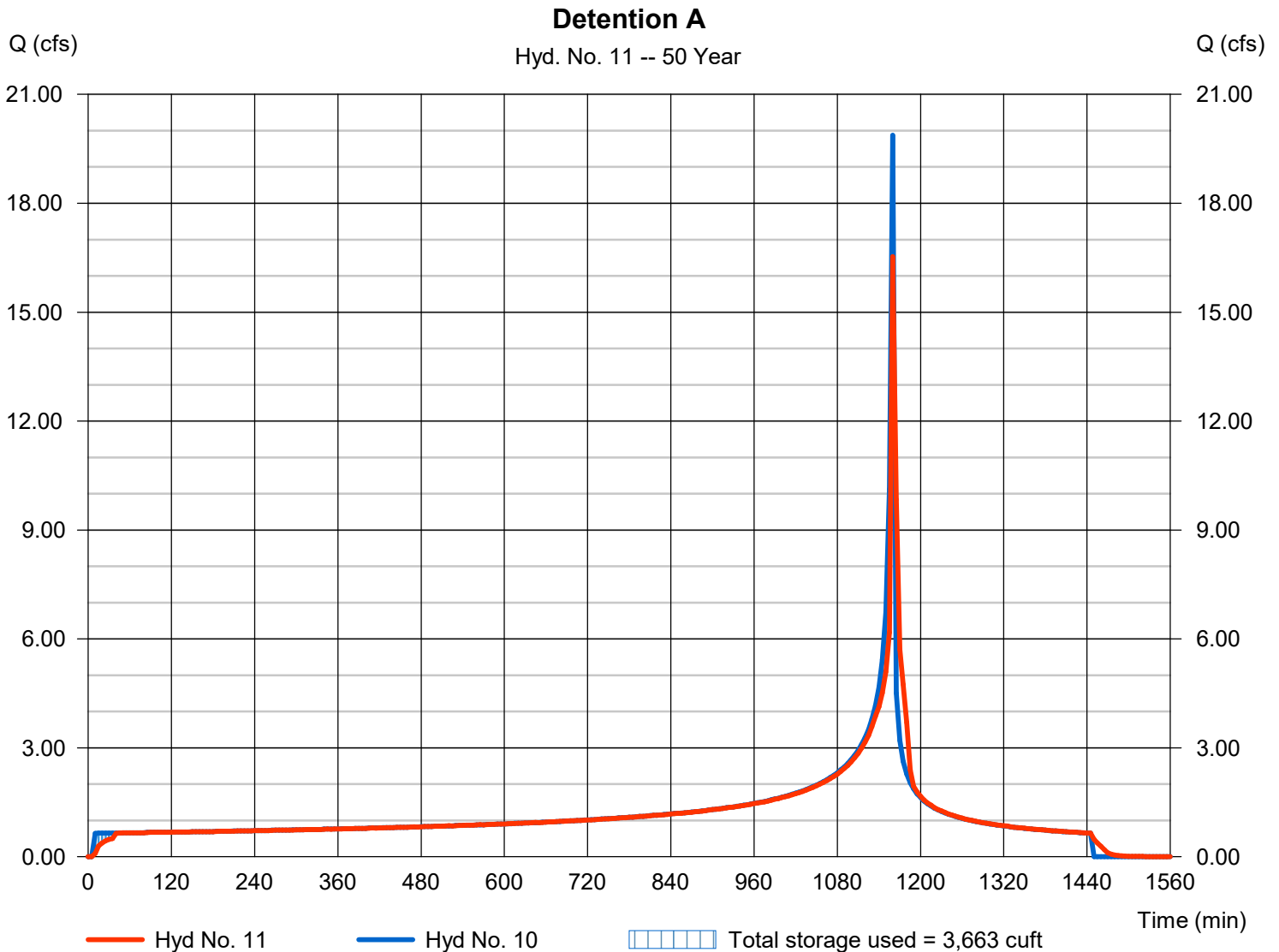
Tuesday, 07 / 21 / 2020

## Hyd. No. 11

Detention A

Hydrograph type	= Reservoir	Peak discharge	= 16.53 cfs
Storm frequency	= 50 yrs	Time to peak	= 1160 min
Time interval	= 5 min	Hyd. volume	= 107,512 cuft
Inflow hyd. No.	= 10 - Prop DA-1_50yr	Max. Elevation	= 52.00 ft
Reservoir name	= Detention A	Max. Storage	= 3,663 cuft

Storage Indication method used.



## Pond No. 3 - Detention B

### Pond Data

UG Chambers -Invert elev. = 49.81 ft, Rise x Span = 5.00 x 5.00 ft, Barrel Len = 160.00 ft, No. Barrels = 2, Slope = 0.10%, Headers = No

### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	49.81	n/a	0	0
0.52	50.33	n/a	269	269
1.03	50.84	n/a	566	835
1.55	51.36	n/a	705	1,540
2.06	51.87	n/a	783	2,323
2.58	52.39	n/a	820	3,143
3.10	52.91	n/a	820	3,963
3.61	53.42	n/a	783	4,746
4.13	53.94	n/a	705	5,451
4.64	54.45	n/a	566	6,016
5.16	54.97	n/a	268	6,284

### Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 12.00	0.00	0.00	0.00
Span (in)	= 12.00	0.00	0.00	0.00
No. Barrels	= 1	0	0	0
Invert El. (ft)	= 49.81	0.00	0.00	0.00
Length (ft)	= 1.00	0.00	0.00	0.00
Slope (%)	= 1.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

### Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 4.00	0.00	0.00	0.00
Crest El. (ft)	= 53.51	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

### Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	49.81	0.00	---	---	---	0.00	---	---	---	---	---	0.000
0.52	269	50.33	0.27 oc	---	---	---	0.00	---	---	---	---	---	0.265
1.03	835	50.84	1.04 oc	---	---	---	0.00	---	---	---	---	---	1.044
1.55	1,540	51.36	3.80 oc	---	---	---	0.00	---	---	---	---	---	3.805
2.06	2,323	51.87	4.73 ic	---	---	---	0.00	---	---	---	---	---	4.729
2.58	3,143	52.39	5.45 ic	---	---	---	0.00	---	---	---	---	---	5.453
3.10	3,963	52.91	6.09 ic	---	---	---	0.00	---	---	---	---	---	6.092
3.61	4,746	53.42	6.67 ic	---	---	---	0.00	---	---	---	---	---	6.670
4.13	5,451	53.94	7.20 ic	---	---	---	3.73	---	---	---	---	---	10.93
4.64	6,016	54.45	7.70 ic	---	---	---	12.22	---	---	---	---	---	19.91
5.16	6,284	54.97	8.16 ic	---	---	---	23.50	---	---	---	---	---	31.66

# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

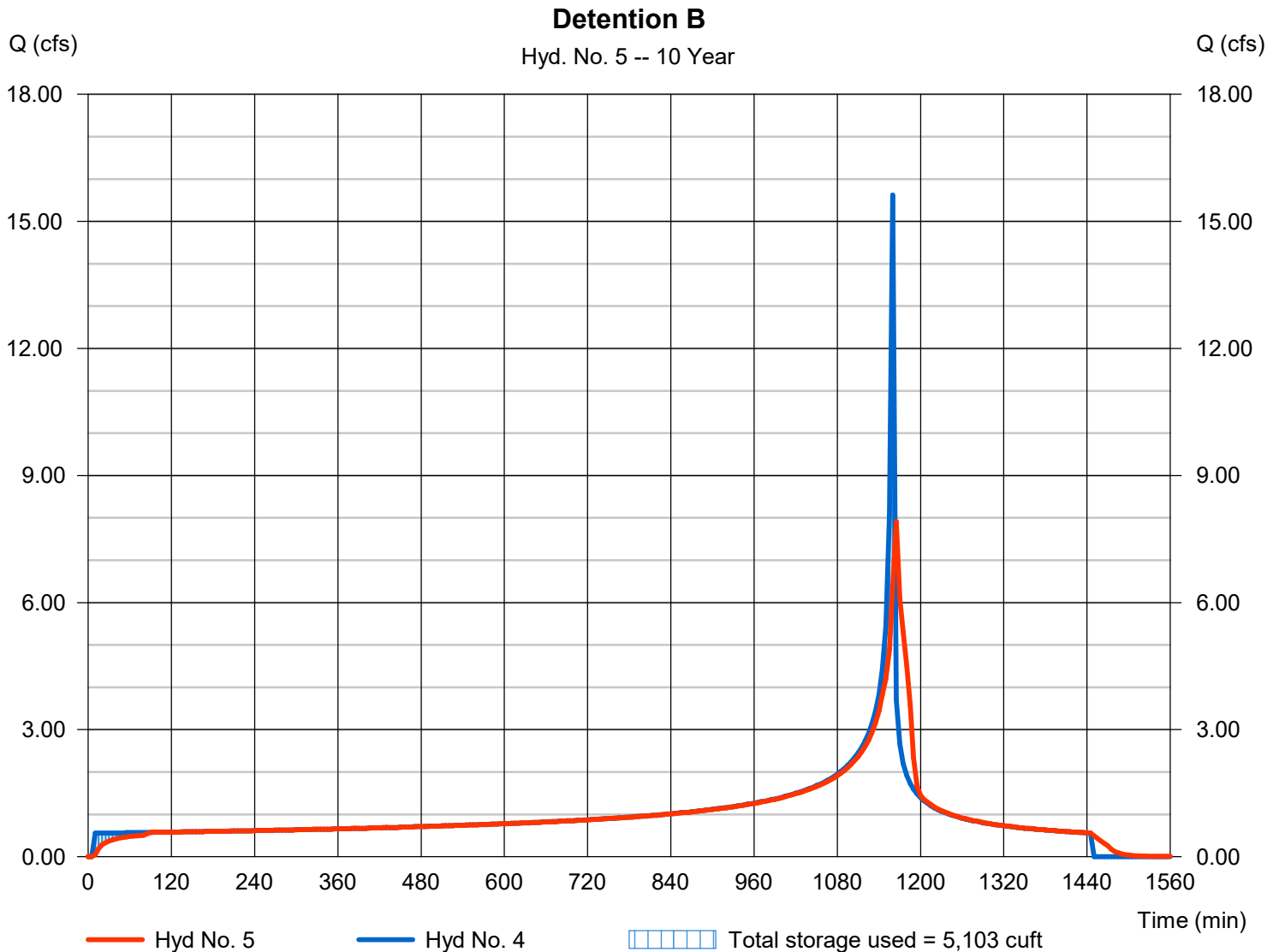
Tuesday, 07 / 21 / 2020

## Hyd. No. 5

Detention B

Hydrograph type	= Reservoir	Peak discharge	= 7.909 cfs
Storm frequency	= 10 yrs	Time to peak	= 1165 min
Time interval	= 5 min	Hyd. volume	= 91,197 cuft
Inflow hyd. No.	= 4 - Prop DA-2_10yr	Max. Elevation	= 54.20 ft
Reservoir name	= Detention B	Max. Storage	= 5,103 cuft

Storage Indication method used.



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

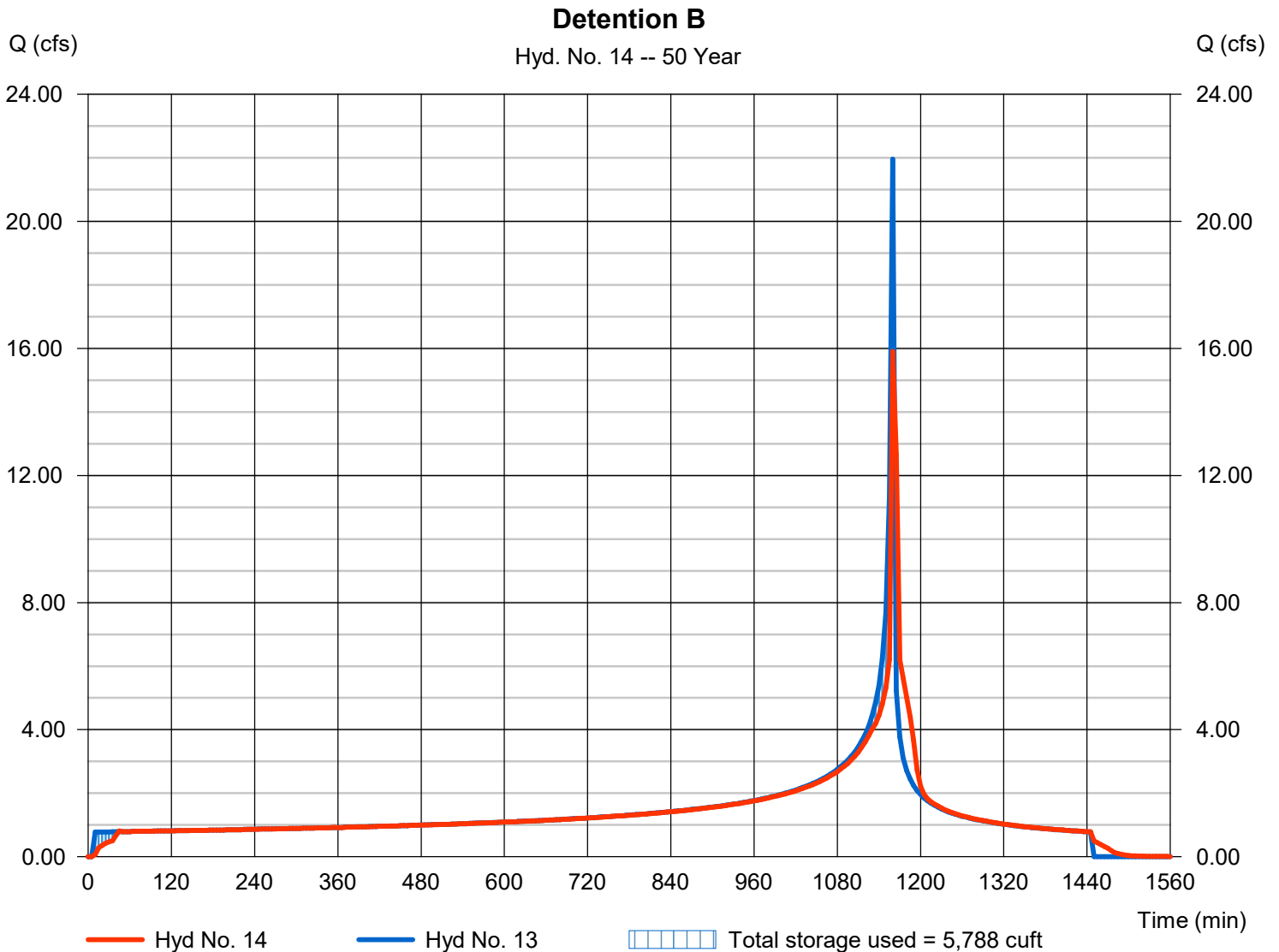
Tuesday, 07 / 21 / 2020

## Hyd. No. 14

Detention B

Hydrograph type	= Reservoir	Peak discharge	= 15.90 cfs
Storm frequency	= 50 yrs	Time to peak	= 1160 min
Time interval	= 5 min	Hyd. volume	= 127,962 cuft
Inflow hyd. No.	= 13 - Prop DA-2_50yr	Max. Elevation	= 54.97 ft
Reservoir name	= Detention B	Max. Storage	= 5,788 cuft

Storage Indication method used.





## **SECTION 5.0**

Storm Drain Master Plan  
Tributary Areas Q and Pipe Capacity Data

